

**STORM SEWER COMPUTATIONS
 FOR
 YORKVILLE SUBDIVISION**

DESIGN DATA

County: **Racine** Design Storm: **10** yr Storm Duration: **10** min DESIGN INTENSITY (I): **5.04** in/hr Intensity calculated using SEWRPC IDF equations.

| STRUCTURE DATA | | | DRAINAGE AREA AND FLOW DATA | | | | PIPE DATA | | | | PIPE CAPACITY INFORMATION | | | | | ELEVATIONS | | |
|----------------|-----------------------------|----------------------|--|--------------------------|-------------------------|-----------------------|-------------|---------------|-----------|---------------------|---|------------------|------------------|-----------------------|---------------------|------------|-----------|-------------|
| Notes | Upstream Structure | Downstream Structure | Flow is determined by Rational Method Q = CIA | | | | Length (ft) | Diameter (in) | Slope (%) | Manning Coefficient | Pipe capacity is determined by Manning's Equation Q = 1.486/n AR ^{2/3} S ^{1/2} | | | | | Rim/Toc Up | Invert Up | Invert Down |
| | | | Individual Acres A | Individual Coefficient C | Individual Flow Q (cfs) | Cumulative Flow (cfs) | | | | | Required Drop (ft) | Actual Drop (ft) | Percent Full (%) | Actual Velocity (fps) | Max. Capacity (cfs) | | | |
| | CULVERT 1 | | 16.40 | 0.23 | 19.01 | 19.01 | 108.00 | 24 | 0.65 | 0.013 | 0.76 | 0.70 | 92% | 6.59 | 19.62 | 725.00 | 722.20 | 721.50 |
| | CULVERT 2 | | 12.60 | 0.21 | 13.34 | 13.34 | 82.00 | 24 | 0.80 | 0.013 | 0.28 | 0.66 | 62% | 6.88 | 21.77 | 726.70 | 722.16 | 721.50 |
| | CULVERT 3 | | 0.80 | 0.46 | 1.85 | 1.85 | 45.00 | 12 | 0.60 | 0.013 | 0.12 | 0.27 | 63% | 3.77 | 2.97 | 726.70 | 723.47 | 723.20 |
| | CULVERT 4 | | 8.80 | 0.24 | 10.64 | 10.64 | 50.00 | 24 | 0.50 | 0.013 | 0.11 | 0.25 | 62% | 5.45 | 17.21 | 725.70 | 722.65 | 722.40 |
| | AREA TO BYPASS SWALE | | 13.00 | 0.20 | 13.10 | 13.10 | 1.00 | 24 | 0.50 | 0.013 | 0.00 | 0.01 | 73% | 5.68 | 17.21 | | | |

CULVERT DESIGN SUMMARY

All 4 culverts are design to handle the 10-year storm even under no surcharge conditions.

Culvert 1 will overtop the new roadway and continue west under larger events.

Culvert 2 needs to be designed to handle the 100-year storm without over topping the backslope of the ditch where it would flow undetained and not tributary to the pond. As shown on the 100-year flow sheet, the culvert is design to handle the 100-year without surcharge.

Culvert 3 needs to be designed to handle the 100-year storm without over topping the backslope of the ditch and having off site water come into the middle pond. As shown on the 100-year flow sheet, the culvert is designed to handle the 100-year event without surcharge and will not over top into the roadside ditch.

Culvert 4 will overtop the new roadway and continue west under larger events. Note that it will also handle the 100-year event.

The bypass swale is designed to carry the 100-year event

**STORM SEWER COMPUTATIONS
 FOR
 YORKVILLE SUBDIVISION**

100-YEAR FLOWS

DESIGN DATA

County: **Racine** Design Storm: **100** yr Storm Duration: **10** min DESIGN INTENSITY (I): **7.65** in/hr Intensity calculated using SEWRPC IDF equations.

| STRUCTURE DATA | | | DRAINAGE AREA AND FLOW DATA | | | | PIPE DATA | | | | PIPE CAPACITY INFORMATION | | | | | ELEVATIONS | | | | |
|----------------|----------------------|----------------------|--|--------------------------|-------------------------|-----------------------|-------------|---------------|-----------|---------------------|---|------------------|------------------|-----------------------|---------------------|------------|-----------|-------------|--|--|
| Notes | Upstream Structure | Downstream Structure | Flow is determined by Rational Method Q = CIA | | | | Length (ft) | Diameter (in) | Slope (%) | Manning Coefficient | Pipe capacity is determined by Manning's Equation Q = 1.486/n AR ^{2/3} S ^{1/2} | | | | | Rim/Toc Up | Invert Up | Invert Down | | |
| | | | Individual Acres A | Individual Coefficient C | Individual Flow Q (cfs) | Cumulative Flow (cfs) | | | | | Required Drop (ft) | Actual Drop (ft) | Percent Full (%) | Actual Velocity (fps) | Max. Capacity (cfs) | | | | | |
| | CULVERT 1 | | 16.40 | 0.23 | 28.86 | 28.86 | 108.00 | 24 | 0.65 | 0.013 | 1.76 | 0.70 | Surcharge | --- | 19.62 | 725.00 | 722.20 | 721.50 | | |
| | CULVERT 2 | | 12.60 | 0.21 | 20.24 | 20.24 | 82.00 | 24 | 0.80 | 0.013 | 0.66 | 0.66 | 88% | 7.34 | 21.77 | 726.70 | 722.16 | 721.50 | | |
| | CULVERT 3 | | 0.80 | 0.46 | 2.82 | 2.82 | 45.00 | 12 | 0.60 | 0.013 | 0.28 | 0.27 | 90% | 4.00 | 2.97 | 726.70 | 723.47 | 723.20 | | |
| | CULVERT 4 | | 8.80 | 0.24 | 16.16 | 16.16 | 50.00 | 24 | 0.50 | 0.013 | 0.26 | 0.25 | 89% | 5.80 | 17.21 | 725.70 | 722.65 | 722.40 | | |
| | AREA TO BYPASS SWALE | | 13.00 | 0.20 | 19.89 | 19.89 | | | | | | | | | | | | | | |

100-YEAR FLOW FOR SWALE ANALYSIS

CULVERT 2, 3 AND 4 WILL PASS THE 100-YEAR

Channel Report

Trapezoidal

Bottom Width (ft) = 1.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 735.00
Slope (%) = 0.70
N-Value = 0.025

Highlighted

Depth (ft) = 1.09
Q (cfs) = 19.90
Area (sqft) = 5.84
Velocity (ft/s) = 3.41
Wetted Perim (ft) = 9.99
Crit Depth, Yc (ft) = 0.98
Top Width (ft) = 9.72
EGL (ft) = 1.27

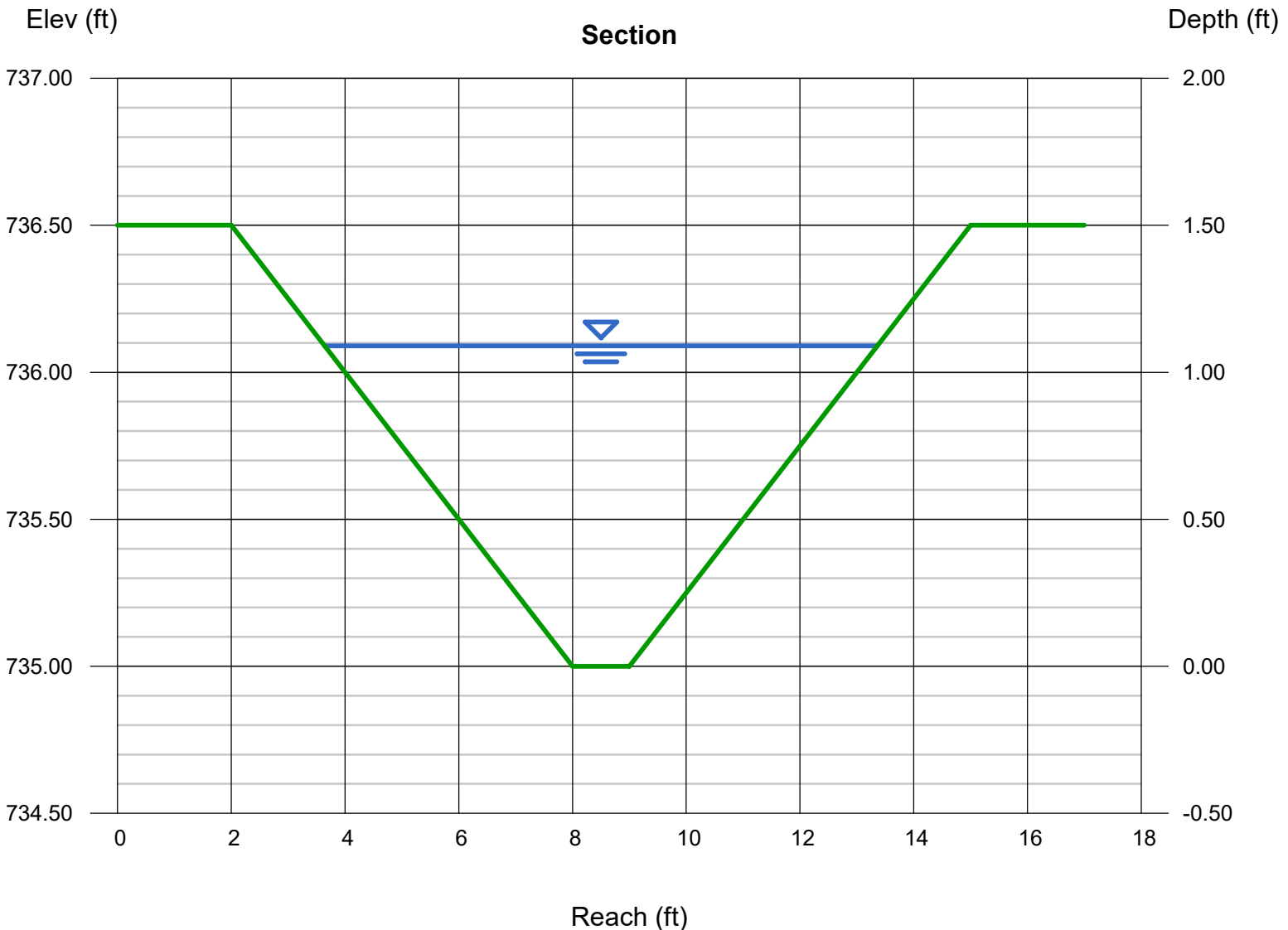
Calculations

Compute by:
Known Q (cfs)

Known Q
= 19.90

FROM 100-YEAR
SPREADSHEET

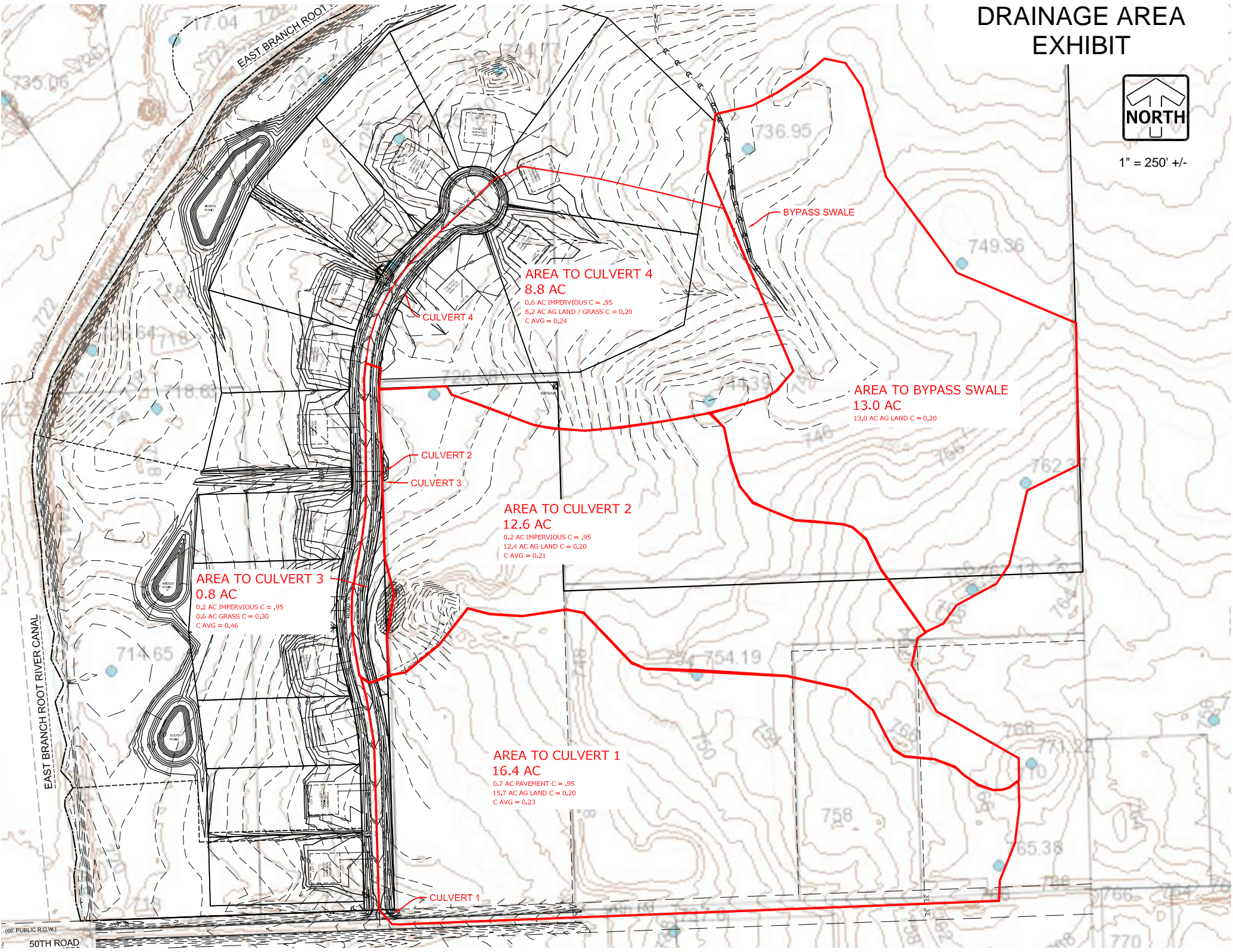
AN 18" DEEP SWALE / BERM WITH
MIN 1' BOTTOM WIDTH WILL
CONVEY THE 100-YEAR EVENT



DRAINAGE AREA EXHIBIT



1" = 250' +/-



AREA TO CULVERT 4
8.8 AC
0.6 AC IMPERVIOUS C = .95
8.2 AC AG LAND / GRASS C = 0.20
C AVG = 0.24

AREA TO BYPASS SWALE
13.0 AC
13.0 AC AG LAND C = 0.20

AREA TO CULVERT 2
12.6 AC
0.2 AC IMPERVIOUS C = .95
12.4 AC AG LAND C = 0.20
C AVG = 0.21

AREA TO CULVERT 3
0.8 AC
0.2 AC IMPERVIOUS C = .95
0.6 AC GRASS C = 0.30
C AVG = 0.46

AREA TO CULVERT 1
16.4 AC
0.7 AC PAVEMENT C = .95
15.7 AC AG LAND C = 0.20
C AVG = 0.23